

**NATIONAL UNIVERSITY OF SCIENCE AND TECHNOLOGY
 FACULTY OF INDUSTRIAL TECHNOLOGY
 DEPARTMENT OF CIVIL AND WATER ENGINEERING
 BACHELOR OF ENGINEERING (HONOURS) DEGREE
 PART III FIRST SEMESTER EXAMINATIONS – JANUARY 2011**

HYDRAULICS – TCW 3101

Instructions:

Answer any 4 questions. All questions carry equal marks.

Total marks: 100

Time: 3 Hours

QUESTION 1

- a. A drainage pump having a tapered suction pipe, discharges water out of a sump. The pipe diameters at the inlet and at the upper end are 1 m and 0.5 m respectively. The free water surface in the sump is 2 m above the center of the inlet and the pipe is laid at a slope 1 (vertical): 4 (along pipeline). The pressure at the top end of the pipe is 0.25 m mercury (density = 13600 kg/m³, viscosity of water = 1x10⁻³ Ns/m²) below atmosphere and it is known that the loss of head due to friction between the two sections is 1/10 of the velocity head at the top section. Compute the discharge through the pipe, taking a length of 20 m. (10 marks)
- b. A pipeline with a constant diameter of 0.3 m turns through an angle of 60°. The centerline of pipe does not change elevation. The discharge through the pipeline is 0.1 m³/s of water and the pressure at the bend is 30 m of water. Calculate the magnitude and direction of the resultant force on the pipe. (15 marks)

QUESTION 2

- a. Two reservoirs, the water levels in which are at elevations 180 m and 150 m respectively, are connected by a pipe 3 km long, 600 mm diameter and friction factor 0.00625. The elevation of the ground along the pipeline is given in Table 2a. Assuming a rounded inlet and an abrupt outlet calculate the discharge. Find the maximum depth of the pipeline below ground if the absolute pressure therein is not to fall below 3 m of water. **Table 2a**

| | | | | | |
|---------------|-----|-----|-----|------|------|
| Distance (m) | 0 | 150 | 300 | 1800 | 3000 |
| Elevation (m) | 175 | 165 | 190 | 140 | 147 |

(15 marks)

- b. A pipeline 10 km long, 300 mm in diameter and with roughness size 0.03 mm, conveys water from a reservoir (top water level 850 m above datum) to a water

treatment plant (inlet water level 700 m above datum). Assuming that the reservoir remains full estimate the discharge, using the Colebrook-White formula and the Moody diagram. (10 marks)

QUESTION 3

- a. Derive the general differential equation for gradually varied flows. (5 marks)
- b. A trapezoidal channel having a Manning’s $n = 0.022$ with 3.5-m bottom width, side 1: 2, bed slope of 0.012, discharge $40 \text{ m}^3/\text{s}$ of fresh water from a reservoir. Determine the water surface profile to within 2% of normal depth. (20 marks)

QUESTION 4

- a. Differentiate between pumps and turbines (6 marks)
- b. An existing pump, having the characteristics shown in Table 4b, is used to pump raw sewage to a treatment plant through a static lift of 20 m. A pipeline 10 km long is to be used. Taking minor losses totaling $10v^2/2g$, pipe roughness of 0.15 mm select a suitable pipe size to achieve a discharge of 60 l/s. Calculate the power consumption. (10 marks)

Table 4b

| | | | | | | | | |
|------------------------|----|------|------|------|------|------|------|------|
| Discharge (l/s) | 0 | 10 | 20 | 30 | 40 | 50 | 60 | 70 |
| Total head (m) | 45 | 44.7 | 43.7 | 42.5 | 40.6 | 38.0 | 35.0 | 31.0 |
| Overall efficiency (%) | - | 35 | 50 | 57 | 60 | 60 | 53 | 40 |

- c. Why is the consideration of cavitation important in pump-pipeline systems design? (2 marks)
- d. If the phenomenon of cavitation is observed in a particular pump, with a cavitation parameter = 0.08, operating at sea level and the pump delivers $0.42 \text{ m}^3/\text{s}$ of water, determine the gauge pressure and the velocity head at the inlet. The dynamic head is 85 m and the suction pipe diameter is 30 cm. (7 marks)

QUESTION 5

- a. Given a wide rectangular channel of width 5 m, maximum depth 2 m, discharge $10 \text{ m}^3/\text{s}$. The normal depth is 1.25 m. Determine the depth of flow downstream of a section in which the bed rises by 0.2 m over a distance of 1 m. (10 marks)
- b. A long rectangular channel 3 m wide carries a discharge of $15 \text{ m}^3/\text{s}$. The channel slope is 0.004 and the Manning’s coefficient 0.01. At a certain point in the channel where the flow reaches the normal depth,
 - i. determine the state of flow

- ii. if the hydraulic jump occurs, what is the sequent depth at the jump?
- iii. estimate the energy loss through the jump.

List of equations

Colebrook-White equation:

$$1/\lambda^{1/2} = -2\log [k/3.7D + 2.51/(\text{Re } \lambda^{1/2})]$$

Colebrook-White - Darcy-Weisbach equation

$$v = -2(2gDs_f)^{1/2} \cdot \log[k/3.7D + 2.51v/D(2gDs_f)^{1/2}]$$

Hazen-Williams equation

$$v = 0.85C_{HW}R_h^{0.63}S^{0.54}$$